

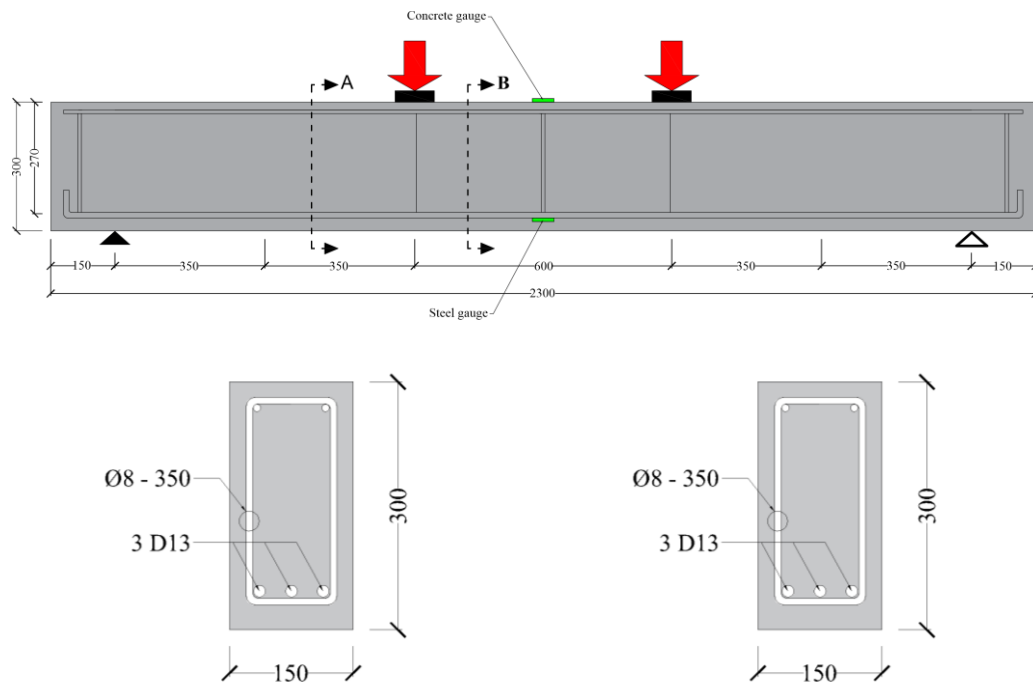
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Lampiran 1 Perhitungan Kapasitas Lentur dan Kapasitas Geser Balok Beton Bertulang



Data Balok

- Panjang Balok (L) : 2300 mm
- Tinggi Balok (h) : 300 mm
- Lebar Balok (b) : 150 mm
- Jarak serat tekan ke titik berat tulangan tarik (d) : 264 mm
- Kuat tekan beton ($f'c$) : 21.1 N/mm²
- Kuat leleh tulangan geser (f_y) : 336.78 N/mm²
- Kuat leleh tulangan tarik (f_y) : 384.82 N/mm²
- Luas tulangan balok
 - Tulangan tekan (A_s') : 100.48 mm²
 - Tulangan tarik (A_s) : 602.88 mm²

a. Kapasitas Lentur Balok (M_n)

$$C_s + C_s = T_s$$

$$(0,85 \times f'c \times a \times b) + (A_s' \times f_y') = A_s \times f_y$$

$$a = \frac{(A_s \times f_y) - (A_s' \times f_y')}{0,85 \times f'_c \times b}$$

$$= \frac{(602,88 \times 384,82) - (100,48 \times 336,78)}{0,85 \times 21,1 \times 150}$$

$$a = 74,459 \text{ mm}$$

$$M_n = T_s \times \left(d - \frac{a}{2} \right)$$

$$M_n = A_s \times f_y \times \left(d - \frac{a}{2} \right)$$

$$M_n = 602,88 \times 384,82 \times \left(264 - \frac{74,459}{2} \right)$$

$$= 51,450 \text{ kNm}$$

Maka,

$$P_n = \frac{M_n}{0,35}$$

$$P_n = \frac{51,450}{0,35}$$

$$= \mathbf{147 \text{ kN}}$$

b. Kapasitas Geser

$$V_c = \frac{1}{6} \sqrt{f'_c} \cdot b_w \cdot d$$

$$= \frac{1}{6} \sqrt{21,1} \cdot 150 \cdot 259$$

$$V_c = 29742 \quad (\mathbf{29,742 \text{ kN}})$$

$$V_s = \frac{A_v \cdot f_y \cdot d}{s}$$

$$V_s = \frac{100,48 \times 384,82 \times 259}{350}$$

$$V_s = 28,613 \text{ N} \quad (\mathbf{28,613 \text{ kN}})$$

Jadi kuat geser nominal

$$V_n = V_c + V_s$$

$$V_n = 29,742 \text{ kN} + 28,613 \text{ kN} = 58,355 \text{ kN}$$

Sehingga

$$P_n = 2V_n$$

$$P_n = 2 \times 58,355$$

$$P_n = \mathbf{116.71 \text{ kN}}$$

Berdasarkan hasil perhitungan analitis menunjukkan $P_{\text{nlentur}} > P_{\text{ngeser}}$. Hal ini menunjukkan bahwa balok beton bertulang mengalami kegagalan geser.

Lampiran 2 Perhitungan Kapasitas Geser Balok SBG

Data balok :

- Panjang Balok (L) : 2300 mm
- Tinggi Balok (h) : 300 mm
- Lebar Balok (b) : 150 mm
- Lebar GFRP (w_f) : 700 mm
- Tinggi efektif NFRP (d_{fv}) : 264 mm
- Jarak serat tekan ke titik berat tulangan tarik (d) : 264 mm
- Jarak serat tekan ke titik tulangan tekan (d') : 32 mm
- Kuat tekan beton (f'_c) : 21.1 N/mm²
- Modulus elastisitas beton; ($E_c = 4700 \cdot f'_c{}^{0.5}$) : 21589.33 N/mm²
- Kuat leleh tulangan geser (f_y) : 336.78 N/mm²
- Jarak antar tulangan geser (s) : 350 mm
- Faktor reduksi kuat lentur (Θ) : 1,00
- Faktor reduksi kekuatan GFRP (ψ_f) : 0,85
- Tulangan Tekan (A_s') : 100.48 mm²
- Tulangan Tarik (A_s) : 602.88 mm²

Ketebalan per lembaran (t_f)	: 1.33 mm
Kuat tarik <i>ultimate</i> (f^*f_u)	: 460 N/mm ²
Regangan <i>ultimate</i> (ε^*f_u)	: 0,02 mm/mm
Modulus elastisitas (E_f)	: 20900 N/mm ²
Jumlah lapisan GFRP/ AbFRP (n)	: 1,00
Sudut pemasangan GFRP/ AbFRP	: 90 mm
Jarak pemasangan GFRP/ AbFRP	: 700 mm

Data Material Tulangan Geser :

Kuat leleh tulangan (f_y)	: 336.78 N/mm ²
Regangan leleh (ε_y)	: 0,00168390 mm/mm
Modulus elastisitas (E_s)	: 200000 N/mm ²

Syarat kuat geser :

$$V_u \leq \Phi V_n$$

Dimana,

V_u = Gaya geser terfaktor pada penampang

V_n = Kuat geser nominal

$$V_n = V_c + V_s$$

Dimana,

V_c = Kuat geser beton

V_s = Kuat geser tulangan sengkang

Kekuatan Geser Beton:

$$V_u = \Phi V_c + \Phi V_s$$

$$V_c = \frac{1}{6} \sqrt{f'_c} \cdot b_w \cdot d$$

$$= \frac{1}{6} \sqrt{21.1} \cdot 150 \cdot 264$$

$$V_c = 30316.93 \text{ N} \quad (30.32 \text{ kN})$$

Kekuatan Geser Tulangan :

$$V_s = \frac{A_v \cdot f_y \cdot d}{s} \quad A_v = 100,48$$

$$V_s = \frac{100,48 \times 336.78 \times 264}{350}$$

$$V_s = 25524.77 \text{ N} \quad (25.52 \text{ kN})$$

Maka kuat geser nominal

$$V_n = V_c + V_s$$

$$= 30.32 + 25.52$$

$$V_n = 55.84 \text{ kN}$$

Kekuatan Geser Perkuatan GFRP:

Berdasarkan persamaan yang diberikan ACI 440, kapasitas geser pada balok beton bertulang yang diperkuat dengan FRP *Composite* dapat dihitung dengan persamaan:

$$\Phi V_n = \Phi (V_c + V_s + \Psi V_f)$$

ACI 440 menjelaskan faktor reduksi untuk kapasitas geser pada balok beton bertulang dengan tiga sisi yang diberi perkuatan FRP sebagai berikut:

$$V_n = 0,75 (V_c + V_s + 0,85 V_f)$$

Terdapat dua kemungkinan kegagalan atau kehancuran yang terjadi pada perkuatan geser yang disumbangkan FRP pada balok beton bertulang, yaitu kegagalan akibat retak atau patahnya FRP dan kegagalan akibat lepasnya rekatan FRP dengan balok beton bertulang.

Untuk perencanaan perkuatan geser berdasarkan kegagalan akibat retaknya (*fractur mechanisme*) FRP, ACI *Committee Report 440* memberikan persamaan:

$$V_f = \frac{A_f f_{fe} (\sin \beta + \cos \beta) d_f}{s_f}$$

– Regangan efektif FRP

$$\varepsilon_{fe} = K_v \quad \varepsilon_{fu} \leq 0,004$$

– Tegangan perkuatan FRP

$$f_{fe} = \varepsilon_{fe} \cdot E_f$$

– Luas perkuatan FRP

$$A_f = 2 n \cdot t_f \cdot w_f$$

Dari persamaan regangan efektif FRP (ε_{fe}), koefisien reduksi dapat dihitung menggunakan persamaan:

$$K_v = \left(\frac{k_1 k_2 L_e}{1190 \varepsilon_{fu}} \right)$$

$$K_1 = \left(\frac{f'c}{27} \right)^{2/3}$$

$$K_2 = \left(\frac{d_f - 2L_e}{d_f} \right) \text{ Untuk rekatan pada kedua sisi}$$

$$\left(\frac{d_f - L_e}{d_f} \right) \text{ Untuk rekatan yang berbentuk U}$$

$$L_e = \frac{23300}{(n \cdot t_f \cdot E_f)^{0,58}}$$

Menentukan Regangan Efektif dari Perkuatan FRP

$$\varepsilon_{fe} = K_v \quad \varepsilon_{fu} \leq 0,004$$

Dari persamaan regangan efektif FRP (ε_{fe}), koefisien reduksi dapat dihitung menggunakan persamaan:

$$K_v = \left(\frac{k_1 k_2 L_e}{1190 \varepsilon_{fu}} \right)$$

Dimana:

$$K_1 = \left(\frac{f_{tc}}{27} \right)^{2/3} = \left(\frac{21.1}{27} \right)^{2/3} = 0,85$$

$$K_2 = \left(\frac{d_f - L_e}{d_f} \right) = \text{untuk rekatan berbentuk U}$$

$$\text{Dimana, } L_e = \frac{23300}{(n \cdot t_f \cdot E_f)^{0,58}}$$

$$L_e = \frac{23300}{(1 \times 1,33 \times 20900)^{0,58}} = 61.64$$

Maka,

$$K_2 = \left(\frac{d_{fv} - L_e}{d_{fv}} \right) = \left(\frac{264 - 61.64}{301,00} \right) = 0.77$$

Maka,

$$\begin{aligned} K_v &= \left(\frac{k_1 k_2 L_e}{1190 \varepsilon_{fu}} \right) = \left(\frac{0,85 \times 0,77 \times 61.64}{1190 \times 0.01} \right) \\ &= 0,225 \leq 0,750 \end{aligned}$$

Maka Regangan Efektif (ε_{fe}):

$$\begin{aligned} \varepsilon_{fe} &= K_v \times \varepsilon_{fu} \leq 0,004 \\ &= 0,2246 \times 0,015 = 0,00337 \leq 0,004 \end{aligned}$$

Menentukan Tegangan Efektif dari Perkuatan FRP

$$\begin{aligned} f_{fe} &= \varepsilon_{fe} \cdot E_f \\ &= 0.0033685 \times 20900 \\ &= 70.40 \end{aligned}$$

$$V_f = \frac{A_{fv} f_e (\sin a + \cos) d_{fv}}{s_f}$$

$$A_f = 2 \times 1 \times 1.33 \times 700$$

$$V_f = \frac{1862 \times 70.40 \times (\sin 90 + \cos 90) \cdot 264.0}{700}$$

$$\begin{aligned}V_f &= 49051.613 \text{ N} \\ &= 49.05 \text{ kN}\end{aligned}$$

Maka Kuat Geser Total adalah:

$$\begin{aligned}V_n &= V_c + V_s + V_f \\ &= 30.32 + 25.52 + 49.05 \\ V_n &= 104.89 \text{ kN}\end{aligned}$$