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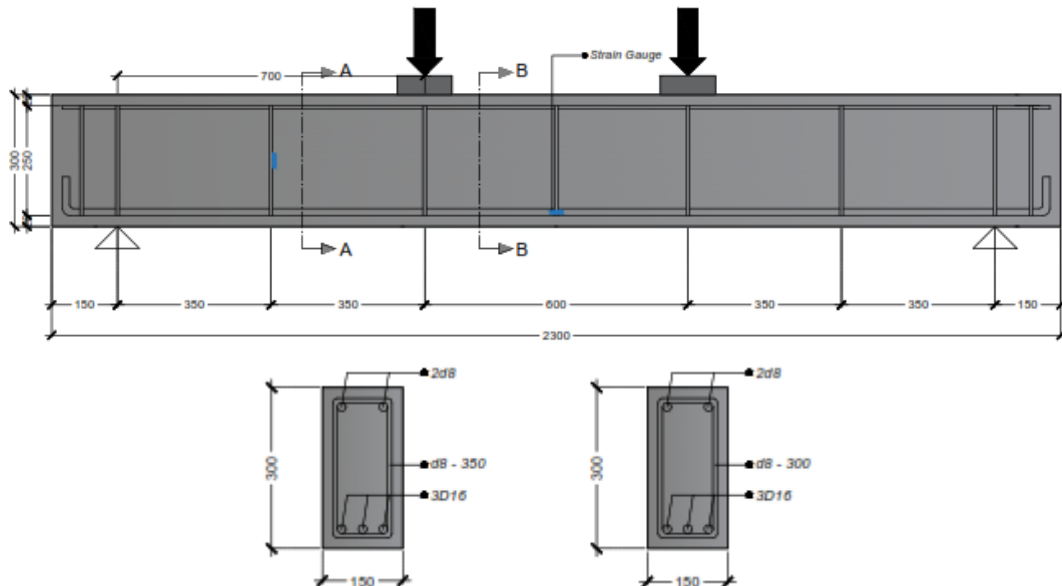
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Perhitungan Kapasitas Lentur dan Kapasitas Geser Balok Beton Bertulang



Data Balok :

- Panjang Balok (L) = 2300 mm
- Tinggi Balok (h) = 300 mm
- Lebar Balok (b) = 150 mm
- Jarak serat tekan ke titik berat tulangan tarik (d) = 264 mm
- Kuat tekan beton (f'_c) = 21.1 MPa
- Kuat leleh tulangan longitudinal
 - Tulangan tekan (f_y') = 336.75 MPa
 - Tulangan Tarik (f_y) = 384.82 MPa
- Luas tulangan balok
 - Tulangan tekan (A_s') = 100.53 mm²

- Tulangan tarik (A_s) = 603.19 mm²

a. Kapasitas Lentur (M_n)

$$C_c + C_s = T_s \dots\dots\dots (7)$$

$$(0.85 \times f'_c \times a \times b) + (A'_s \times f'_y) = A_s \times f_y \dots\dots\dots (8)$$

$$a = \frac{(A_s \times f_y) - (A'_s \times f'_y)}{0.85 \times f'_c \times b} \dots\dots\dots (9)$$

Dimana:

C_c = Gaya tekan pada beton (kN)

C_s = Gaya tekan pada tulangan (kN)

T_s = Jumlah gaya total dari tulangan tarik (kN)

a = Tinggi blok tekan equivalen (mm)

Diperoleh:

$$a = \frac{(A_s \times f_y) - (A'_s \times f'_y)}{0.85 \times f'_c \times b}$$

$$a = \frac{(603.19 \text{ mm}^2 \times 384.82 \text{ MPa}) - (100,53 \text{ mm}^2 \times 336.75 \text{ MPa})}{0.85 \times 21.1 \text{ MPa} \times 150 \text{ mm}}$$

$$a = 73.70$$

$$M_n = A_s \times f_y \times \left(d - \frac{a}{2}\right) \dots\dots\dots (10)$$

$$M_n = A_s \times f_y \times \left(d - \frac{a}{2}\right) + A'_s \times f'_s (d - d') \dots\dots\dots (11)$$

Diperoleh:

$$Mn = A_s \times f_y \times \left(d - \frac{a}{2}\right)$$

$$Mn = 603.19 \text{ mm}^2 \times 384.82 \text{ MPa} \times \left(264 \text{ mm} - \frac{73.70}{2}\right)$$

$$Mn = 52.73 \text{ kNm}$$

Sehingga,

$$P_n = \frac{M_n}{0.35}$$

$$P_n = \frac{M_n}{0.35}$$

$$P_n = \frac{52.73 \text{ kNm}}{0.35 \text{ m}}$$

$$P_n = 150.65 \text{ kN}$$

b. Kapasitas Geser (V_n)

$$V_c = \frac{1}{6} \sqrt{f'_c} \cdot b_w \cdot d \dots\dots\dots(12)$$

Diperoleh:

$$V_c = \frac{1}{6} \sqrt{f'_c} \cdot b_w \cdot d$$

$$V_c = \frac{1}{6} \sqrt{21.1 \text{ kN}} \cdot 150 \text{ mm} \cdot 264 \text{ mm}$$

$$V_c = 30.32 \text{ kNm}^2$$

$$V_s = \frac{A_s \times f_y \times d}{s} \dots\dots\dots (13)$$

Diperoleh:

$$V_s = \frac{A_s \times f_y \times d}{s}$$

$$V_s = \frac{100.53 \text{ mm}^2 \times 384.82 \text{ MPa} \times 264 \text{ mm}}{350 \text{ mm}}$$

$$V_s = 29.18 \text{ kNm}^2$$

Jadi, kuat geser nominal (V_n):

$$V_n = V_c + V_s$$

$$V_n = 30.32 \text{ kNm}^2 + 29.18 \text{ kNm}^2$$

$$V_n = 59.50 \text{ kN}$$

Sehingga,

$$P_n = 2V_n \dots\dots\dots (14)$$

$$P_n = 2 \times 59.50 \text{ kN}$$

$$P_n = 118.99 \text{ kN}$$

Perhitungan Kebutuhan Angkur Baut dan Jarak Antar Baut

Diketahui :

- Beban maksimum GM-W-200 (p_u) = 168.40 kN
- Beban maksimum GM-W-300 (p_u) = 153.21 kN
- Jumlah bidang geser (m) = 2
- Baut dengan ulir pada bid. Geser (r_1) = 0.4
- Kuat tarik putus baut (f_u^b) = 370 MPa
- Luas baut (A_b) = 50.24 mm²
- Untuk semua jenis lubang(n) = 2.4
- Tebal pelat (t_p) = 15 mm
- Diameter lubang baut (d_b) = 14 mm
- Kuat tekan rata-rata geopolimer (f_u) = 21.9 MPa

a. Tahanan nominal baut ditinjau terhadap kuat geser

$$R_n = m \cdot r_1 \cdot f_u^b \cdot A_b \dots\dots\dots (15)$$

Dimana :

R_n = Tahanan nominal baut (kN)

Diperoleh :

$$R_n = m \cdot r_1 \cdot f_u^b \cdot A_b$$

$$R_n = 2 \cdot 0.4 \cdot 370 \text{ MPa} \cdot 50.24 \text{ mm}^2$$

$$R_n = 14871.04 \text{ N} \rightarrow 14.87 \text{ kN}$$

b. Tahanan nominal baut ditinjau terhadap kuat tumpu

$$R_n = n \cdot d_b \cdot t_p \cdot f_u \dots \dots \dots (16)$$

$$R_n = 2.4 \cdot 14 \cdot 15 \cdot 21.9$$

$$R_n = 11037.60 \text{ N} \rightarrow 11.07 \text{ kN}$$

Tahanan nominal baut yang digunakan adalah nilai tahanan nominal baut terkecil. Sehingga nilai R_n yang digunakan yaitu tahanan nominal baut yang ditinjau terhadap kuat tumpu sebesar 11.04 kN

1. Kebutuhan jumlah baut

$$n_b = \frac{P_u/2}{R_n} \dots \dots \dots (17)$$

Dimana:

n_b = Jumlah angkur baut

P_u = Beban maksimum benda uji (kN)

R_n = Tahanan nominal baut (kN)

Diperoleh:

- Benda uji GM-W-200

$$n_b = \frac{P_u/2}{R_n}$$

$$n_b = \frac{168.40/2}{11.04}$$

$$n_b = 7.63 \rightarrow 8 \text{ Buah}$$

- Benda uji GM-W-300

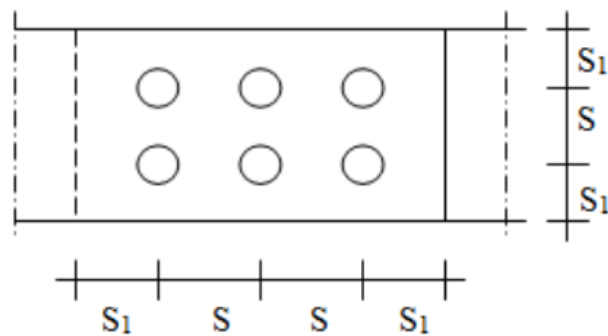
$$n_b = \frac{P_u/2}{R_n}$$

$$n_b = \frac{153.21/2}{11.04}$$

$$n_b = 6.93 \rightarrow 7 \text{ Buah}$$

2. Jarak antar angkur baut

Berdasarkan SNI 1729:2002 Terdapat beberapa persyaratan penempatan angkur baut pada pelat diantaranya:

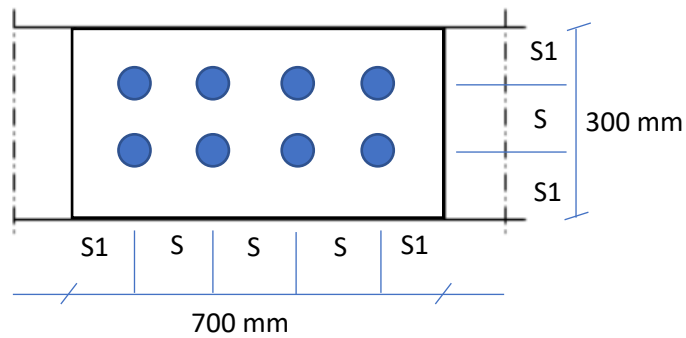


$$3d_b < S < 15t_p \text{ atau } 200 \text{ mm}$$

$$1,5d_b < S_1 < (4t_p + 100\text{mm}) \text{ atau } 200 \text{ mm}$$

- Jarak antar pusat lubang baut harus diambil tidak kurang dari 3 kali diameter nominal baut dan jarak antara baut tepi dengan ujung pelat harus sekurang-kurangnya 1.5 diameter nominal baut.
- Jarak maksimum antar pusat lubang baut tak boleh melebihi $15t_p$ (dengan t_p adalah tebal pelat lapis tertipis dalam sambungan) atau 200 mm, sedangkan jarak tepi maksimum harus tidak melebihi $(4t_p + 100\text{mm})$ atau 200 mm.

Direncanakan:



$$3d_b < S < 15t_p \text{ atau } 200 \text{ mm}$$

$$(3 \cdot 12) < S < (15 \cdot 15) \text{ atau } 200 \text{ mm}$$

$$36 \text{ mm} < S < 225 \text{ mm atau } 200 \text{ mm}$$

Maka, diambil nilai S sebesar 200 mm

$$1.5d_b < S_1 < (4t_p + 100 \text{ mm}) \text{ atau } 200 \text{ mm}$$

$$1.5 \cdot 12 < S_1 < (4 \cdot 15 + 100 \text{ mm}) \text{ atau } 200 \text{ mm}$$

$$18 \text{ mm} < S_1 < (160 \text{ mm}) \text{ atau } 200 \text{ mm}$$

Maka, diambil nilai S_1 sebesar 50 mm